6/5/20

PUTNAM COUNTY DEPARTMENT OF HEALTH
DIVISION OF ENVIRONMENTAL HEALTH SERVICES

CONSTRUCTION PERMIT FOR SEWAGE TREATMENT SYSTEM

PERMIT # K-03-20

Located at 475 Pudding street — Town or Village Kent Carmel

Subdivision name N/A — Subd. Lot # N/A — Tax Map 31 Block 1 Lot 32

Date Subdivision Approved N/A — Renewal — Revision —

Owner/Applicant Name Vitiello — Date of Previous Approval 9/26/91

Mailing Address 475 Pudding Street, Carmel, NY — Zip 10512

Amount of Fee Enclosed $500.00

Building Type Residential — Lot Area 32.58 — No. of Bedrooms 9 — Design Flow GPD 1,350

Fill Section Only ____ **Depth** ____ **Volume** ____
PCHD NOTIFICATION IS REQUIRED WHEN FILL IS COMPLETED

Separate Sewerage System to consist of 1,000 and 2,000 gallon septic tank and 568 L.F. of 2' WIDE ABSORPTION TRENCHES and 100% Expansion (New). (Use current SDS for Apartment with 100% expansion provided.

Other Requirements: 2500 GALLON DOSING TANK

To be constructed by TBD — Address —

Water Supply: ____ Public Supply From — — Address —

or: ____ Private Supply Drilled by — — Address —
WELL IS EXISTING

I represent that I am wholly and completely responsible for the design and location of the proposed system(s) and that the separate sewage treatment system described above will be constructed as shown on the approved amendment thereto and in accordance with the standards, rules and regulations of the Putnam County Department of Health, and that on completion thereof a "Certificate of Construction Compliance" satisfactory to the Public Health Director will be submitted to the Department, and a written guarantee will be furnished the owner, his successors, heirs or assigns by the builder, that said builder will place in good operating condition any part of said sewage treatment system during the period of two (2) years immediately following the date of the issuance of the approval of the Certificate of Construction Compliance of the original system or any repairs thereto.

Signed: John M. Watson — John M Watson P.E. X — RA ____ Date 6/12/20

Address WHITE ENGINEERING, SURVEYING & LANDSCAPE ARCHITECTURE, P.C. 36 GARRETT PLACE, CARMEL NEW YORK 10512 — License # 119501

APPROVED FOR CONSTRUCTION: This approval expires two years from the date issued unless construction of the sewage treatment system has been completed and inspected by the PCHD and is revocable for cause or may be amended or modified when considered necessary by the Public Health Director. Any revision or alteration of the approved plan requires a new permit. Approved for discharge of domestic sanitary sewage only.

By: ____ Title: APHE Date: 6/5/20

White copy - HD File; Yellow copy - Building Inspector; Pink copy - Owner; Orange copy - Design Professional

Form CP-97

Vitello Erosion Control Bond

Eric Schlobohm, PE <ESchlobohm@insite-eng.com>
Fri 9/25/2020 11:09 AM
To: Planning Kent <planningkent@townofkentny.gov>

3 attachments (2 MB)
3359_001.pdf; Erosion Control Bond.pdf; PerformanceBondFV.pdf;

TOWN OF KENT NOTICE

THIS EMAIL IS FROM AN EXTERNAL SENDER!
DO NOT click links, DO NOT open attachments, DO NOT forward if you were not expecting this email or if it seems suspicious in any way! REMEMBER: NEVER provide your user ID or password to anyone for any reason!

Vera,

As discussed attached please find the bank check, signed bond and bond estimate. We request being on the next available Town Board Agenda.

Thanks and have a great weekend,

Eric

INSITE
ENGINEERING, SURVEYING & LANDSCAPE ARCHITECTURE, P.C.

Eric M. Schlobohm, PE,
Associate
Senior Project Engineer

INSITE ENGINEERING, SURVEYING & LANDSCAPE ARCHITECTURE, P.C.
3 Garrett Place
Carmel, New York 10512
(845) 225-9690 x119
(845) 225-9717 Fax
www.insite-eng.com

HOLD DOCUMENT UP TO THE LIGHT TO VIEW TRUE WATERMARK

citibank
Citibank, N.A.

OFFICIAL CHECK

HOLD DOCUMENT UP TO THE LIGHT TO VIEW TRUE WATERMARK

115411350

FOR 00010 FA# 070 $0.00 ONL PE
028-02 Ck. Ser.# 115411350

DATE 09/15/20

52-29
311

PAY ****SIX THOUSAND ONE HUNDRED TWENTY-FOUR DOLLARS****

TO THE ORDER OF ****TOWN OF KENT****

$6,124.00

Drawer: Citibank, N.A.

NAME OF REMITTER
ADDRESS

Citibank, N.A. One Penn's Way
New Castle, DE 19720

BY
AUTHORIZED SIGNATURE

MP

⑈115411350⑈ ⑆031100209⑆ 3876292411⑈

INSITE
ENGINEERING, SURVEYING & LANDSCAPE ARCHITECTURE, P.C.

EROSION CONTROL BOND ESTIMATE

Prepared For
Vitiello Residence
475 Pudding Street
Town of Kent, New York

September 9, 2020

Item	Quantity	Unit Price	Total
Stabilized Construction Entrance	1 Each	$750.00 Each	$750.00
Silt Fence	470 L.F.	$2.00 / L.F.	$940.00
Erosion Control Blanket	410 S.Y.	$1.90 / S.Y.	$779.00
Seeding & Mulching	28,000 S.F.	$0.06 / S.F.	$1,680.00
Temporary Soil Stockpile	3 Each	$575.00 Each	$1,725.00
Rip Rap Outlets and Splash Pads	Lump Sum	$250.00	$250.00
		TOTAL	**$6,124.00**

Insite File #19261.100
Z:\E\19261100 Vitiello\Bonds\ecbond-090420.doc

PERFORMANCE BOND FOR EROSION AND SEDIMENT CONTROL

Frank & Juli Vitiello
475 Pudding Street
Kent, NY 10512
TM: 31.-1-32

Bond given by Frank & Juli Vitiello, 475 Pudding Street, Kent, New York 10512/Tax Map 31.-1-32 ("Obligor") to the Town of Kent, a municipal corporation whose Town Hall is located at 25 Sybil's Crossing, Kent Lakes, New York, 10512 ("Obligee"), dated 9/23/20.

KNOW ALL MEN BY THESE PRESENTS that the Obligor is held and firmly bound unto the Obligee in the sum of $6,124.00, along with an initial inspection fee deposit of $1,000.00 for the payment whereof to the Obligee the said Obligor binds itself, its successors and assigns.

WHEREAS, Obligor has obtained the approvals from the Obligee for land development activity, as that term is defined in Town of Kent Town Code Chapter 66 (the "Code"), on certain real property located in the Town of Kent, in connection with which erosion and sedimentation controls ("Controls") are required and the Erosion and Sediment Control Plan documents shall be required in accordance with the Code in effect as of the date of this Bond; and

WHEREAS, in conjunction with such Steep Slope and Erosion Control Permit Applications, the Obligor has submitted to the Obligee, plans and specifications for the construction of an addition to a single family residence and a detached garage with an apartment on the second level known as the Vitiello Property ("Project Plans") and the Erosion and Sediment Control Plan, prepared by Insite Engineering, 3 Garrett Place, Carmel, NY 10512.

All these plans were reviewed on September 10, 2020 by the Obligee. A conditional approval of land development activity in the nature of a Steep Slope and Erosion Control Permit of plans to construct a single family residence addition and detached garage in an R-80 zoning district. The proposed project is within the NYCDEP East of Hudson watershed and will disturb more than 5,000 SF of land. A Town of Kent Steep Slope and Erosion Control Permit is required as well as coverage under NYSDEC SPDES General Permit for Stormwater Discharges from Construction Activity, GP-0-20-001.

WHEREAS, the amount of this bond is based upon a recommendation by the Planning Board Consulting Engineer to the Planning Board dated September 10, 2020; and

WHEREAS, as condition to the issuance and approval of a Steep Slope and Erosion Control Permit, the Code requires the owner or applicant, prior to construction, to provide the Obligee with a cash escrow, an irrevocable letter of credit or a certified check drawn upon a national or state bank or other cash equivalent, which guarantees satisfactory completion of the Erosion and Sediment Control Plan, which security shall remain in full force and effect until the Obligor is released from liability by the Town, provided that such a period shall not be less than two years from the date of final acceptance or such other certification that the erosion and sedimentation controls have been completed in accordance with approved Project Plans.

WHEREAS, the Planning Board of the Town of Kent has granted the Erosion Control Permit subject to the posting of two checks made out to the Town of Kent, one in the amount of $6,124.00 as a performance guarantee to be deposited into an escrow account pending the completion of the project for which the erosion control measures are necessary; and a second check in the amount of $1,000.00 as the initial inspection fee to be held in escrow and used to fund inspections by the Town's consultants during construction and delivered to:

The Kent Planning Board
25 Sybil's Crossing
Kent, NY 10512

No funds may be withdrawn from the said escrow account until a resolution has been duly made by the **Town Board** of **The Town of Kent**, authorizing such surrender or cashing.

In the event the erosion control work allowed shall not have been duly completed by **FRANK & JULI VITIELLO,** as per the conditions and specifications of the **Planning Board** of **The Town of Kent**, the **Town Board** shall have the right to withdraw the aforesaid escrow monies ($6,124.00 performance guarantee and remaining money left in the inspection fee of $1,000.00 or as replenished) and complete the required work for **FRANK & JULI VITIELLO;** with full use of said sums as the Town requires;

Upon full completion of the work allowed pursuant to the conditions and specifications heretofore imposed by the **Planning Board** of **The Town of Kent**, by **FRANK & JULI VITIELLO,** the aforesaid escrow monies ($6,124.00 performance guarantee and remaining money left in the inspection fee of $1,000.00) after the work has been completed shall be returned or refunded to **FRANK & JULI VITIELLO, 475 Pudding Street, Kent, NY 10512;**

This bond may not be assigned or transferred without the prior written approval of the **Planning Board** and **Town Board** of **The Town of Kent.**

The applicant hereby expressly authorizes the **Town of Kent**, its agents, employees, engineers, consultants and/or planners to enter upon the Owner's/Applicant's property for the purpose of inspecting the erosion control system installed and the site work being performed in accordance with the approved plans, provided that the **Town of Kent** provides at least 24 hours notice to **FRANK & JULI VITIELLO**;.

Dated: 9/23, 2020

FRANK VITIELLO

By: _____
(signature)

(print/type signatory's name)
Frank Vitiello

JULI VITIELLO

By: _____
(signature)

(print/type signatory's name)
Juli Vitiello

Owner/Obligee, Frank & Juli Vitiello
(print/type signatory's title)

STATE OF New York)

) ss.:

COUNTY OF New York)

On the 23rd day of September, 2020, before me, the undersigned, a notary public in and for said state, personally appeared Frank Vitiello and Juli Vitiello, personally known to me or proved to me on the basis of satisfactory evidence to be the individual(s) whose name(s) is(are) subscribed to the within instrument and acknowledged to me that he/she/they executed the same in his/her/their capacity(ies), and that by his/her/their signature(s) on the instrument, the individual(s), or the person upon behalf of which the individual(s) acted, executed the instrument.

NOTARY PUBLIC

PATRICK L MCROBERTS
NOTARY PUBLIC-STATE OF NEW YORK
No. 01MC6386633
Qualified in New York County
My Commission Expires 01-28-2023

INSITE
ENGINEERING, SURVEYING & LANDSCAPE ARCHITECTURE, P.C.

Received
mo/date/year

SEP 28 2020

Planning Department
Town of Kent
September 24, 2020

Town of Kent Planning Board
Kent Town Centre
25 Sybil's Crossing
Kent Lakes, New York 10512

RE: Vitiello Residence
475 Pudding Street
Kent, NY
Tax Map No. 31.-1-32

Dear Chairman Tolmach and Members of the Board:

Enclosed please find fourteen (14) copies of the following:

- Project Plans (4 sheets), last revised September 21, 2020. (5 full scale and 9 reduced scale)
- Bank Check #115411350 for $6,124.00 Erosion Control Bond (1).
- Signed Erosion Control Bond.(3)
- Check #376 for $1,000.00 Inspection Fee (1).
- Erosion Control Bond Estimate.
- NYSDEC Notice of Intent (NOI).(4)
- Geotechnical Report. (4)

In response to the September 9, 2020 comment memo from Julie S. Mangarillo, P.E., we offer the following:

1. A signed NOI and a MS4 Acceptance Form have been enclosed herewith for coverage under GP-0-20-001.

4. GP-0-20-001 notes have been added to Drawing SP-2.

10. A check for inspection fees is enclosed herewith.

1. No response required.

2. The Town of Kent has been noted as the MS4 in the NOI.

3. A label has been added to the rip rap outlets for the roof drain and footing drain on Drawing SP-2.

4. The bond estimate has been revised to include an erosion control blanket and rip rap pipe outlets as requested. It was referred to the Town Board at the 9/10/20 Planning Board meeting.

5. The recent storm and its aftermath brought down many trees and branches. In the clean up afterwards, the applicant brought in new topsoil and intends to stabilize the area along with planting new specimen trees, commemorating loved ones in their family. This area and the new temporary dirt trail adjacent to the existing garage are approximately 0.18 acres and will

be shown on the plan. The total disturbance remains less than 1 acre. The soil stockpile is to be used to repair pot holes in the driveway as a result of storm damage.

6. The existing tree line has been added to Drawing SL-1 and the proposed tree line has been added to Drawing SP-1. A note stating no tree shall be removed outside the limits of the disturbance has been added to Drawing SP-2.
7. Stabilization of the dirt road and landscape area has been noted on Drawing SP-2.
8. An email noting no objection to the septic plans from the Putnam County Department of Health was previously forwarded. The formal approval will be forwarded once received.

In response to the September 9, 2020 comment memo from Bruce Barber, we offer the following:

1. It is understood that no wetland permit is required.
2. It is understood that the action is a Type II action under SEQRA.
3. The existing tree line has been added to Drawing SL-1 and the proposed tree line has been added to Drawing SP-1. A note stating no tree shall be removed outside the limits of the disturbance has been added to Drawing SP-2.
4. General Note #4 on Drawing OP-1 has been revised as requested.
5. The project is anticipated to be built in two phases with the addition to the main house as the first phase of work. (They are to be filed as two separate Building Permits) The rock hammering for the addition foundation/footings and basement excavation is anticipated to take approximately 10 to 15 days of machine work, depending on the hardness of the rock encountered. The garage building has been moved 10 feet, thus the boring that was taken is now out of the building envelope. Based on the rocky conditions being more favorable as a result, as well as that the structure is slab on grade, this scope is anticipated to take approximately 10 days of rock hammering. Note there will be no rock hammering or excavation work on weekends. All work will be undertaken during normal business hours as allowed by the Building Department.
6. An email noting no objection to the septic plans from the Putnam County Department of Health was previously forwarded. The formal approval will be forwarded once received.
7. Comments from the Town Engineer are addressed above.

Should you have any questions or comments regarding this information, please feel free to contact our office.

Very truly yours,

INSITE ENGINEERING, SURVEYING & LANDSCAPE ARCHITECTURE, P.C.

By: John Watson (es)
John M. Watson, P.E.
Senior Principal Engineer

JMW/EMS/amk

Enclosures:
cc: Carol Kurth

Insite File No. 19261.100

Town of Kent Planning Board
25 Sybil's Crossing
Tel: 845-225-7802

email: planningkent@townofkentny.gov
Kent, NY 10512
Fax: 845-306-5283

Memorandum

DATE: October 1, 2020

TO: Finance Department

CC:

FROM: Vera Patterson

RE: Vitiello TM: 31.-1-32

Please find attached Citibank Check 376 dated 9/15/20 in the amount of $1,000.00 to be deposited into the escrow account noted above for final inspection.

Thanks very uch.:

RECEIPT DATE 10/1/20 No. 537114

$1,000.00

RECEIVED FROM Vitiello

One Thousand and 00/100 DOLLARS

FOR RENT

FOR Rec. Fee TM: 31.-1-32 374

ACCOUNT		CASH
PAYMENT		CHECK
BAL. DUE		MONEY ORDER
		CREDIT CARD

FROM V. Patterson TO F. Vitiello

BY

3-11

JULI VITIELLO
FRANK D. VITIELLO

376

Date 9/15/20

Pay to the order of Town of Kent $ 1,000.-

One Thousand 00/100 Dollars

citibank Citigold

CITIBANK, N.A.

For Inspection escrow

:021000089: 6796231599 0376

Vitiello Property 31.-1-32

Date		Deposits	Fees	Balance
7/30/2020	Check #315	1,000.00		1,000.00
08/25/20	Rohde-29657-2478		(28.00)	972.00
09/03/20	Cornerstone - 20-0903 Aug 2020		(687.50)	284.50
09/03/20	Rohde-29657-2525		(840.00)	(555.50)
09/24/20	Citibank Ck 372 - escrow	1,500.00		944.50
10/01/20	Citibank Ck 376 - Inspection Fee	1,000.00		1,944.50

Town of Kent Planning Board
25 Sybil's Crossing
Tel: 845-225-7802

email: planningkent@townofkentny.gov
Kent, NY 10512
Fax: 845-306-5283

Memorandum

DATE: **October 1, 2020**

TO: **Finance Department**

CC:

FROM: **Vera Patterson**

RE: **Vitiello TM: 31.-1-32**

Please find attached Citibank Check 376 dated 9/15/20 in the amount of $1,000.00 to be deposited into the escrow account noted above for final inspection.

Thanks very uch.:

DOWN TO EARTH
CONSULTING, LLC
GEOTECHNICAL AND ENVIRONMENTAL ENGINEERING

GEOTECHNICAL ENGINEERING REPORT
VITIELLO RESIDENCE ADDITION AND GARAGE
475 PUDDING STREET
CARMEL, NEW YORK

Prepared for:

e2 Engineers
488 Montauk Ave.
New London, Connecticut 06320

Prepared by:

Down To Earth Consulting, LLC
122 Church Street
Naugatuck, Connecticut 06770

File No. 0020-047.00
July 2020

DOWN TO EARTH
CONSULTING, LLC
GEOTECHNICAL AND ENVIRONMENTAL ENGINEERING

July 3, 2020
File No. 0020-047.00

Scott Erricson, P.E.
e2 Engineers
488 Montauk Ave.
New London, Connecticut 06320

Via email: scott.erricson@e2engineers.com

Re: Geotechnical Engineering Report
Vitiello Residence Addition and Garage
Carmel, New York

Dear Mr. Erricson:

Down To Earth Consulting, LLC (DTE) is pleased to submit this geotechnical engineering report for the proposed addition and garage that will be constructed at the Vitiello residence located at 475 Pudding Street in Carmel, New York. We appreciate this opportunity to work with you. Please call if you have any questions.

Sincerely,

Down To Earth Consulting, LLC

Daniel F. LaMesa, P.E.
Principal

TABLE OF CONTENTS

APPENDICES

1.0 INTRODUCTION

This report provides geotechnical design and construction recommendations for the proposed addition and garage that will be constructed at the Vitiello residence located at 475 Pudding Street in Carmel, New York. Refer to Figures 1 and 2 (in Appendix 1) for the approximate site and proposed structure locations, respectively.

We understand that a new one-story addition will be constructed off the western and southern ends of the exiting house and a new two-story garage will be build off the southern end of the driveway. The first floor (approximate Elevation (El.) 101.5) of the addition will be about 3,000 square feet with a crawl space below the western section and full basement level (approximate El. 90.5) below the southern section. The garage will have an approximate footprint of 1,575 square feet with a slab-on-grade at approximate El. 98.5 and second story at approximate El. 110. Associated sidewalks, drainage structures, utilities, and landscape areas will also be constructed. Foundation and slab loads were not available at the time this report was prepared.

Our geotechnical engineering services included: reviewing project plans, observing test borings, characterizing subsurface conditions within the project limits, performing geotechnical engineering analyses, and providing geotechnical design and construction recommendations for the proposed structures. Our services were performed in accordance with our June 4, 2020, proposal.

Our recommendations are based on allowable stress design methods and the 2020 New York State Residential Building Code. Elevations stated herein are in feet and based on the Vitiello Residence Site Plan, 475 Pudding Street, Carmel, New York, prepared by Carol Kurth Architecture, dated May 19, 2020.

2.0 TEST BORINGS

We observed and logged three test borings (B-1 through B-3) drilled by our subcontractor Associated Borings Co., Inc. on June 23, 2020. Boring locations are depicted on Figure 2 (Appendix 1) and the logs are included in Appendix 2. Borings were located in the field by taping/pacing from existing site features and their elevations were scaled from the above referenced plan. The boring locations and elevations should be considered approximate.

The borings were drilled to explore the soil and groundwater conditions in the project area. Hollow-stem auger drilling methods were used to advance borings to depths of approximately 3 to 11.5 feet (approximate El. 101 to 88.5) below existing grades. All borings were terminated on Inferred Bedrock/Bedrock.

Representative soil samples were obtained for soil classification by split barrel sampling procedures in general accordance with ASTM D-1586. The split-spoon sampling procedure utilizes a standard 2-inch O.D. split-barrel sampler that is driven into the bottom of the boring with a 140-pound hammer falling a distance of 30 inches. The number of blows required to advance the sampler the middle 12-inches of a normal 24-inch penetration is recorded as the Standard Penetration Resistance Value (N). The blows (i.e., "N-Value") are indicated on the boring logs at their depth of occurrence and provide an indication of the relative consistency of the material.

Bedrock coring was performed at Boring B-1 using a NQ double-tube core barrel. Descriptions of the rock cores are presented on the logs in addition to Recovery and Rock Quality Designation (RQD). Recovery is defined as the length of core obtained expressed as a percentage of the total length cored. RQD is the total length of core pieces, 4 inches or greater in length, expressed as a percentage of the total length cored. RQD provides an indication of the quality of the rock mass and relative extent of jointing and foliations.

Groundwater levels were measured using a weighted tape in open drill holes or inferred from wet soil samples during drilling.

3.0 SUBSURFACE CONDITIONS

3.1 SUBSURFACE PROFILE

The generalized subsurface profile, as inferred from the subsurface data, consists of about 4 to 12-inches of Topsoil over natural Sand Deposits and Bedrock. An approximate 3-foot layer of uncontrolled Fill was encountered between the Topsoil and Sand at B-1. The Fill was loose and appeared to be reworked native soil from prior site development. The following is a more detailed description of the Natural Sand Deposits and Bedrock encountered at the site:

3.1.1 Sand Deposits

Natural Sand Deposits were present directly below the Topsoil and Fill (if present). This material generally consisted of medium dense, gray/brown, fine to coarse sand with little (10 to 20%) fine Gravel and little (10 to 20%) silt. Cobbles and possible boulders were inferred in this stratum based on drilling behavior.

3.1.2 Inferred Bedrock/Bedrock

Bedrock was confirmed with coring at Boring B-1 and inferred based on auger refusal at Borings B-2 and B-3 from about 2 to 8.4 feet (approximate El. 102 to 94) below the ground surface.

The bedrock core obtained at Boring B-1 was classified as very poor quality, moderately hard, moderately weathered, gray/dark gray, fine to medium grained Granitic Gneiss. The core recovery and RQD was 78% and 15%, respectively.

3.2 GROUNDWATER

Groundwater was not observed in the test borings. Groundwater levels measured in the boreholes may not have had sufficient time to stabilize and should be considered approximate. Groundwater levels will vary depending on factors such as temperature, season, precipitation, construction activity, and other conditions, which may be different from those at the time of these measurements.

4.0 GEOTECHNICAL RECOMMENDATIONS

We offer the following geotechnical design recommendations based on the subsurface conditions encountered at the site, available project information, and proposed construction.

4.1 FOUNDATIONS

4.1.1 Foundation Type and Bearing Strata

We recommend supporting the proposed structures on normal, shallow spread footings. The footings should bear on undisturbed natural Sand Deposits, Bedrock or on Structural Fill (hereinafter specified as Compacted Granular Fill, CGF) over natural Sand and/or Bedrock. Existing Fill, Topsoil, and buried structures (e.g., footings, utilities, etc.) are not suitable for support of foundations. Actual bottom of unsuitable bearing material elevations will vary across the site and should be verified during construction excavation by a DTE representative.

When CGF is used beneath the footings (e.g., in backfill areas), we recommend that it be placed one foot beyond the edge of the footings and at a one horizontal to one vertical slope away and down from the bottom outside edge of the footings. Crushed Stone can be used in place of CGF as it is much easier to compact.

4.1.2 Footing Levels and Sizes

Exterior footings (and footings in unheated areas) should be constructed at a minimum frost depth of 42-inches below proposed site grades or directly on sound bedrock. Interior footings, in heated areas, should be constructed at a minimum depth of 24-inches below proposed top of slab-on-grade level or directly on sound Bedrock. The minimum footing width should be 2 feet.

4.1.3 Allowable Bearing Pressures and Settlement Estimates

We recommend a maximum allowable design bearing pressure of five kips per square foot (ksf) for footings bearing on the recommended bearing materials. Higher pressures can be accommodated on for footings bearing directly on Bedrock should they be desired by the project's structural engineer, in which case DTE should be consulted. Based on the recommended bearing strata and anticipated loads, we anticipate that footings will undergo less than one inch of total settlement and less than a half inch of differential settlement. Settlements will occur as the loads are applied and are expected to be complete at the end of construction.

4.1.4 Drainage

We recommend the use of a perimeter footing drain due to the poorly draining site soils and shallow bedrock. The footing drains should consist of 4-inch diameter perforated PVC pipe, surrounded by 6-inches of Crushed Stone, wrapped in non-woven filter fabric. Cleanouts should be installed in the direction of flow at the beginning of piping runs and consist of 45 degree elbows (90 degree elbows should not be allowed). The drains should be gravity drained to the site drainage system or sump pits with pumps.

4.2 SLABS-ON-GRADE

We recommend placing at-grade slabs over a minimum six-inch-thick base course layer of compacted Crushed Stone or Sand and Gravel placed over the surface of natural soil and/or Bedrock. The subgrade modulus for the recommended subgrades and base course is 250 pounds per cubic inch.

Slab damp-proofing should be installed between the slab and base course, and consist of not less than 6-mil polyethylene with joints lapped at least 6-inches.

4.3 RETAINING WALLS

4.3.1 Backfill and Drainage

We recommend backfilling earth retaining structures with compacted Sand and Gravel and installing footing drains. The drains should consist of 4-inch diameter perforated PVC pipe, surrounded by 6-inches of Crushed Stone, wrapped in non-woven filter fabric. Footing drain inverts should be set flush with or up to 6-inches above bottom of footing levels. The drains should be gravity drained to daylight or the site drainage system.

4.3.2 Lateral Earth Pressures

Walls that are free to rotate at the top and are not braced should be designed to resist an equivalent active static horizontal fluid earth pressure equal to 35 pcf (based on $\varphi' = 34°$, $c = 0$, $Ka = 0.28$, $\delta = 17°$, and $\gamma = 125$ pcf). Braced retaining walls (e.g., crawl space walls, basement walls, etc.) should be designed to resist an equivalent at-rest static horizontal fluid earth pressure equal to 56 pcf (based on $\varphi' = 34°$, $c = 0$, $Ko = 0.45$, and $\gamma = 125$ pcf). This assumes no unbalanced hydrostatic pressures, seismic forces, or surcharge loads (e.g., vehicles).

Due to the limited expected wall movement and depth of footings, we do not recommend the use of passive earth pressures against the base of walls.

4.3.3 Coefficient of Friction

We recommend a maximum coefficient of friction of 0.45 between foundations and the recommending bearing strata.

4.4 SEISMIC DESIGN

The site class is "C" per the Building Code. Based on the standard penetration test results, visual soil classification, and design peak ground acceleration at this locale, the site soils are not susceptible to liquefaction.

5.0 MATERIALS RECOMMENDATIONS

5.1 COMPACTED GRANULAR FILL

Compacted Granular Fill (CGF) for use as structural fill below footings and other structures shall consist of inorganic soil free of clay, loam, ice and snow, tree stumps, roots, and other organic matter; graded within the following limits:

Sieve Size	Percent finer by weight
4-inches	100%
No. 10	30 - 100
No. 40	10 - 90
No. 200	0 - 15

5.2 SAND AND GRAVEL

Sand and Gravel for use below slabs and as retaining wall backfill shall consist of hard, durable sand and gravel; free of ice, clay, shale, roots, sod, rubbish, and other organic matter; graded within the following limits:

Sieve Size	Percent finer by weight
2-inches	100%
1/2-inch	50 - 85
No. 4	40 - 75
No. 40	10 - 35
No. 200	0 - 5

5.3 CRUSHED STONE

Crushed Stone for use around drains or below foundations and slabs shall consist of sound, tough, durable, rock that is graded within the following:

Sieve Size	Percent finer by weight
5/8-inches	100%
1/2-inch	85 - 100
3/8 inch	15 - 45
No. 4	0 - 15
No. 8	0 - 5

5.4 GEOTEXTILE FABRIC

Geotextile fabric placed around crushed stone pipe bedding or used as a separation fabric for crushed stone and soil material should meet the following criteria:

Property	Criteria	Test Method
Grab Strength	min. 80lbs	ASTM D4632
Static (CBR) Puncture	min. 50lbs	ASTM D6241

Trapezoid Tear	min. 25lbs	ASTM D4533
Apparent Opening Size	No. 70-100 U.S. Sieve Size	ASTM D4751

Fabric should be needle-punched non-woven material. Seams should be overlapped a minimum of six inches. During stone placement, the stone drop height should not exceed three feet and equipment traffic should be kept off the fabric until at least 6 to 12 inches of material is placed.

5.5 PLACEMENT AND COMPACTION REQUIREMENTS

The required degree of compaction shall be based on a maximum dry density as determined by a Modified Proctor (ASTM D1557). The degree of compaction for fill placed in various areas shall be as follows:

Placement areas	Minimum degree of compaction
1. Below foundations (footings)	95%
2. Below slabs	92%
3. Against foundation and retaining walls	92%
4. Landscaped areas	90%

Crushed stone is considered to be self-compacting, and would negate the need to run laboratory proctor testing and have field density testing of in-place lifts. The crushed stone should be plate compacted to "chink up" the working surface in lifts. We recommend placing Crushed Stone in maximum 12-inch lifts and compacting the lifts with a minimum of four passes with a vibratory plate compactor weight of a minimum of 1,000 pounds and with a minimum centrifugal force of 10,000 pounds. Recommended loose lift thickness for granular fill and the minimum number of passes of compaction equipment are summarized on the table below.

Extra care should be used when compacting adjacent to walls. Hand-operated rollers or plate compactors weighing not more than 250 pounds should be used within a lateral distance of 5 feet of walls. Where walls are buried on both sides, backfill and compaction should proceed on both sides of the wall so that the difference in top of fill on either side of the wall does not exceed 2 feet.

Hand-operated vibratory plate or light roller in confined areas	4"	6"	8"	6	4
Hand-operated vibratory drum rollers weighing at least 1,000#	6"	8"	10"	6	4
Light vibratory drum roller, minimum dynamic force 3,000# per ft. of drum width	6"	10"	14"	6	4
Medium vibratory drum roller, minimum dynamic force 5,000# per ft. of drum width	8"	12"	18"	6	4
Large vibratory drum roller, minimum dynamic force 8,000# per ft. of drum width	10"	16"	24"	6	4

6.0 CONSTRUCTION RECOMMENDATIONS

6.1 FOOTING PREPARATION

We recommend the use of smooth edged excavator buckets (not back-bladed) to make the final excavations and placing and compacting a four-inch-thick layer of Crushed Stone over footing subgrades for protection. The stone will provide a transition between bedrock and soil subgrades and also protect the subgrades during construction.

The base of footing excavations should be free of water, ice, and loose and frozen soils prior to placing stone and concrete. Should the materials at bearing level become disturbed, the affected materials should be removed prior to placing stone and concrete. Concrete should be placed as soon as possible after excavation and placement of Crushed Stone so that excessive weathering of bearing materials does not occur.

6.2 BEDROCK REMOVAL

Bedrock removal may be required to reach subgrade levels for some of the proposed foundation and slab areas. Boulders and decomposed Bedrock may be able to be removed with an excavator. For sound Bedrock removal, hydraulic splitters, air rams, or other more aggressive methods may be required.

Drilling and blasting may be required for economical removal of the bedrock in some areas. Controlled blasting techniques must be implemented to protect adjacent structures from vibrations and limit risk of over-blast and excessive fracturing (and unnecessary over-excavation and replacement). Bedrock removal specifications should be developed and require blast contractor

submissions and a blasting plan outlining rock drilling, controlled blasting, and vibration monitoring.

6.3 TEMPORARY EXCAVATIONS

The Fill and natural site soils are classified as OSHA Class "C" soil and can be cut at a maximum one vertical to one and a half horizontal (1V:1.5H) slope up to a maximum excavation depth of 20 feet. These maximum slope and excavation depths assume no surcharge load (i.e., existing building footings, construction equipment, etc.) at the top of the excavations or groundwater seepage.

If excavations cannot be sloped in accordance with OSHA requirements and without undermining adjacent structures (e.g. roadway, utilities, etc.), temporary excavation support systems and/or underpinning of existing foundations will be required. Systems should be chosen and installed by the contactor and designed by a Professional Engineer registered in the State of New York.

6.4 TEMPORARY GROUNDWATER CONTROL

We expect that temporary groundwater/storm water control can largely be accomplished by means of shallow trenches and sump pumps, and grading the excavation to low points.

7.0 REVIEW OF FINAL DESIGN, PLANS, AND SPECIFICATIONS

When project plans and specifications are available they should be provided to DTE for review of conformance with our geotechnical recommendations. If any changes are made to the proposed building, the recommendations provided in this report will need to be verified by DTE for applicability.

8.0 CONSTRUCTION QUALITY CONTROL

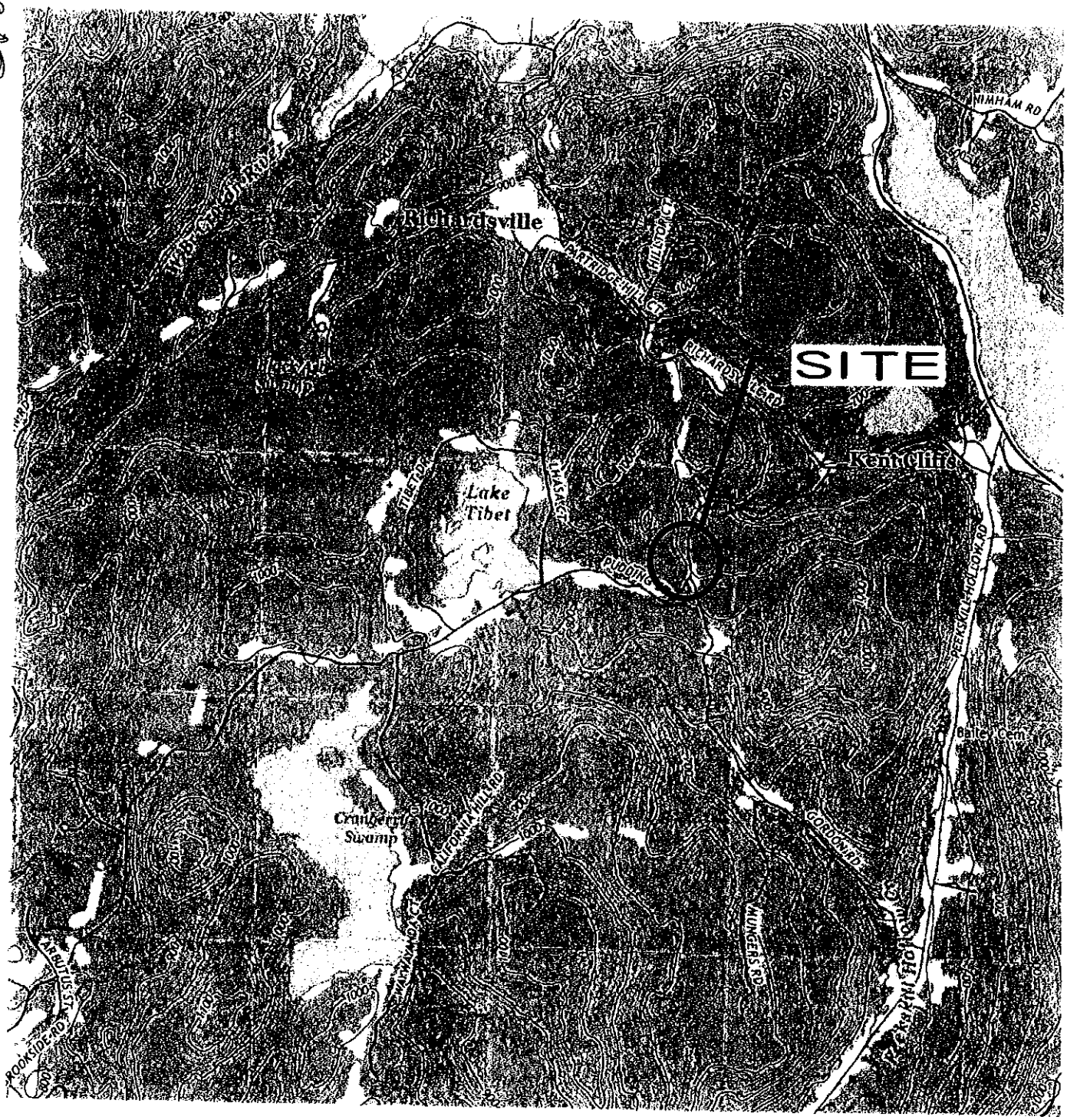
We recommend that DTE make field observations of excavations and foundation preparation to monitor compliance with our recommendations and project specifications. Specifically, we recommend field observation of footing subgrades, removal of unsuitable materials, Fill placement and compaction, and existing Fill improvement (e.g., surface densification) to monitor compliance with project specifications.

9.0 LIMITATIONS

This report is subject to the limitations included in Appendix 3.

APPENDIX 1 -

FIGURES



DOWN TO EARTH CONSULTING, LLC
GEOTECHNICAL AND ENVIRONMENTAL ENGINEERING

122 CHURCH STREET
NAUGATUCK, CONNECTICUT 06770

QUADRANGLE LOCATION

AREA PLAN
VITIELLO RESIDENCE ADDITION AND GARAGE
475 PUDDING STREET
CARMEL, NEW YORK

REFERENCE:
USGS TOPOGRAPHIC QUADRANGLE: OSCAWANA, NY

SCALE 1"= 2,000'

2,000' 1,000' 0 2,000'

DRAWN BY: MF	REVIEWED BY: DFL

PROJECT NO.	0020-047.00
DATE:	7/3/20
FIGURE NO.	1

APPENDIX 2 -

TEST BORING LOGS

DOWN TO EARTH CONSULTING, LLC

PROJECT		BORING NO.	B-1
VITIELLO RESIDENCE ADDITION AND GARAGE		SHEET	1 of 1
475 PUDDING STREET		FILE NO.	0020-047.00
CARMEL, NEW YORK		CHKD. BY	DFL

Boring Co.	Associated Borings Company, Inc.	Boring Location	See Boring Location Plan
Driller	Jamie Lloret	Ground Surface El.	100'±
Logged By	Mateusz Fekieta	Date Start	6/23/2020
		Datum	
		Date End	6/23/2020

Hammer Type:	Safety hammer driven by cathead with a 30 inch drop
Sampler Size:	1-3/8" I.D. Split Spoon
Type Drill Rig:	Truck CME 55
Drilling Method:	3.25-inch I.D. Hollow-Stem Augers

Date	Time	Depth (ft)	Elev.	Stabilization Time
6/23/20	-	-	-	Not Observed

1							
2		S-1	7/24	1 to 3	5-8-9-11		4"± Topsoil
3							Medium dense, brown, fine to coarse SAND, little fine Gravel, little Silt
4		S-2	9/24	3 to 5	13-12-11-11		SAND
5							Medium dense, brown, fine to coarse SAND, little fine Gravel, little Silt
6		S-3	11/16	5 to 6.3	11-13-50/4"		
7		C-1	47/60	6.5 to 11.5		2	Medium dense, brown, fine to coarse SAND, little fine Gravel, little Silt
8						8.5	
9						10.5	Very poor quality, moderatly hard, moderatly weathered, gray/dark gray, fine to medium grained, GRANITIC GNIESS (RQD = 9"/80" = 15%)
10						3.7	BEDROCK
11						3	
12							END OF EXPLORATION AT 11.5 FEET BELOW GROUND SURFACE
13							
14							
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39							
40							

0 to 4 - Very Loose	0 to 2 - Very Soft	Trace = 0 to 10%	1. S denotes split-barrel sampler.
5 to 10 - Loose	3 to 4 - Soft	Little = 10 to 20%	2. ST denotes 3-inch O.D. undisturbed sample.
11 to 30 - Medium Dense	5 to 8 - Medium Stiff	Some = 20 to 35%	3. UO denotes 3-inch Osterberg undisturbed sample.
31 to 50 - Dense	9 to 15 - Stiff	And = 35 to 50%	4. PEN denotes penetration length of sampler.
Over 50 - Very Dense	16 to 30 - Very Stiff		5. REC denotes recovered length of sample.
	Over 30 - Hard		6. SPT denotes Standard Penetration Test.

7. WH denotes weight of hammer
8. WR denotes weight of rods
9. PP denotes Pocket Penetrometer.
10. FVST denotes field vane shear test.
11. RQD denotes Rock Quality Designation.
12. C denotes core run number.

FIELD NOTES: 1) Stratification lines represent approximate boundaries between soil types, transitions may be gradual.
2) Water level readings have been made at times and under conditions stated, fluctuations may occur due to other factors.
3) Auger refusal at 6.4 feet below ground surface inferred bedrock.

DOWN TO EARTH CONSULTING, LLC

PROJECT
VITIELLO RESIDENCE ADDITION AND GARAGE
475 PUDDING STREET
CARMEL, NEW YORK

BORING NO. B-2
SHEET 1 of 1
FILE NO. 0020-047.00
CHKD. BY DFL

Boring Co.	Associated Borings Company, Inc.	Boring Location	See Boring Location Plan
Driller	Jamie Lloret	Ground Surface El.	103'±
Logged By	Mateusz Fekieta	Date Start	6/23/2020
		Datum	
		Date End	6/23/2020

Hammer Type:	Safety hammer driven by cathead with a 30 inch drop
Sampler Size:	1-3/8" I.D. Split Spoon
Type Drill Rig:	Truck
Drilling Method:	2.25-inch I.D. Hollow-Stem Augers

Date	Time	Depth (ft)	Elev.	Stabilization Time
6/23/20	-	-	-	Not Observed

1								
2		S-1	6/24	1 to 3	2-2-4-6		Loose, brown, fine to coarse SAND, little fine Gravel, little Silt	4"± Topsoil FILL
3								
4		S-2	10/24	3 to 5	7-8-10-10		Medium dense, brown, fine to coarse SAND, little fine Gravel, little Silt	
5								
6		S-3	7/24	5 to 7	5-8-8-6		Medium dense, brown, fine to coarse SAND, little fine Gravel, little Silt	SAND
7								
8		S-4	10/16	7 to 8.4	11-19-50/4"		Very dense, brown, fine to coarse SAND, little fine Gravel, little Silt	
9								
10							END OF EXPLORATION AT 8.4 FEET BELOW GROUND SURFACE	INFERRED BEDROCK
11								
12								
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22								
23								
24								
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37								
38								
39								
40								

0 to 4 - Very Loose 5 to 10 - Loose 11 to 30 - Medium Dense 31 to 50 - Dense Over 50 - Very Dense	0 to 2 - Very Soft 3 to 4 - Soft 5 to 8 - Medium Stiff 9 to 15 - Stiff 16 to 30 - Very Stiff Over 30 - Hard	Trace = 0 to 10% Little = 10 to 20% Some = 20 to 35% And = 35 to 50%	1. S denotes split-barrel sampler. 2. ST denotes 3-inch O.D. undisturbed sample. 3. UO denotes 3-inch Osterberg undisturbed sample. 4. PEN denotes penetration length of sampler. 5. REC denotes recovered length of sample. 6. SPT denotes Standard Penetration Test.	7. WH denotes weight of hammer 8. WR denotes weight of rods 9. PP denotes Pocket Penetrometer. 10. FVST denotes field vane shear test. 11. RQD denotes Rock Quality Designation. 12. C denotes core run number.

FIELD NOTES: 1) Stratification lines represent approximate boundaries between soil types, transitions may be gradual.
2) Water level readings have been made at times and under conditions stated, fluctuations may occur due to other factors.
3) Auger chatter at 8 feet below ground surface inferred weathered bedrock, cobbles, and/or boulders.
4) Auger refusal at 8.4 feet below ground surface inferred bedrock.

DOWN TO EARTH CONSULTING, LLC

PROJECT		
VITIELLO RESIDENCE ADDITION AND GARAGE	BORING NO.	B-3
475 PUDDING STREET	SHEET	1 of 1
	FILE NO.	0020-047.00
CARMEL, NEW YORK	CHKD. BY	DFL

Boring Co.	Associated Borings Company, Inc.	Boring Location	See Boring Location Plan		
Driller	Jamie Lloret	Ground Surface El.	104'±	Datum	NAVD 88
Logged By	Mateusz Fekieta	Date Start	6/23/2020	Date End	6/23/2020

Hammer Type:	Safety hammer driven by cathead with a 30 inch drop
Sampler Size:	1-3/8" I.D. Split Spoon
Type Drill Rig:	Truck CME 55
Drilling Method:	3.25-inch I.D. Hollow-Stem Augers

Date	Time	Depth (ft)	Elev.	Stabilization Time
6/23/20	-	-	-	Not Observed

1							12" Topsoil	
2							SAND	
3							END OF EXPLORATION AT 2 FEET BELOW GROUND SURFACE	INFERRED BEDROCK
4								
5								
6								
7								
8								
9								
10								
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0 to 4 - Very Loose	0 to 2 - Very Soft	Trace = 0 to 10%	1. S denotes split-barrel sampler.
5 to 10 - Loose	3 to 4 - Soft	Little = 10 to 20%	2. ST denotes 3-inch O.D. undisturbed sample.
11 to 30 - Medium Dense	5 to 8 - Medium Stiff	Some = 20 to 35%	3. UO denotes 3-inch Osterberg undisturbed sample.
31 to 50 - Dense	9 to 15 - Stiff	And = 35 to 50%	4. PEN denotes penetration length of sampler.
Over 50 - Very Dense	16 to 30 - Very Stiff		5. REC denotes recovered length of sample.
	Over 30 - Hard		6. SPT denotes Standard Penetration Test.

7. WH denotes weight of hammer
8. WR denotes weight of rods
9. PP denotes Pocket Penetrometer.
10. FVST denotes field vane shear test.
11. RQD denotes Rock Quality Designation.
12. C denotes core run number.

FIELD NOTES: 1) Stratification lines represent approximate boundaries between soil types, transitions may be gradual.
2) Water level readings have been made at times and under conditions stated, fluctuations may occur due to other factors.
3) Auger refusalt at about 2 feet below ground surface inferred bedrock.

APPENDIX 3 -

LIMITATIONS

LIMITATIONS

Explorations

1. The analyses and recommendations submitted in this report are based in part upon the data obtained from subsurface explorations. The nature and extent of variations between these explorations may not become evident until construction. If variations then appear evident, it will be necessary to reevaluate the recommendations of this report.

2. The generalized soil profile described in the text is intended to convey trends in subsurface conditions. The boundaries between strata are approximate and idealized and have been developed by interpretations of widely spaced explorations and samples; actual soil transitions are probably more erratic. For specific information, refer to the boring logs.

3. Water level readings have been made in the drill holes at times and under conditions stated on the boring logs. These data have been reviewed and interpretations have been made in the text of this report. However, it must be noted that fluctuations in the level of the groundwater may occur due to variations in rainfall, temperature, and other factors occurring since the time measurements were made.

Review

4. In the event that any changes in the nature, design or location of the proposed structures are planned, the conclusions and recommendations contained in this report shall not be considered valid unless the changes are reviewed and conclusions of this report modified or verified in writing by DTE. It is recommended that this firm be provided the opportunity for a general review of final design and specifications in order that earthwork and foundation recommendations may be properly interpreted and implemented in the design and specifications.

Construction

5. It is recommended that this firm be retained to provide soil engineering services during construction of the earthworks and foundation phases of the work. This is to observe compliance with the design concepts, specifications, and recommendations and to allow design changes in the event that subsurface conditions differ from those anticipated prior to start of construction.

Use of Report

6. This report has been prepared for the exclusive use of e2 Engineers and their design team for specific application to the project noted in this geotechnical report in accordance with generally accepted soil and foundation engineering practices. No other warranty, express or implied, is made.

7. This soil and foundation engineering report has been prepared for this project by DTE. This report is for design purposes only and is not sufficient to prepare an accurate bid. Contractors wishing a copy of the report may secure it with the understanding that its scope is limited to design considerations only.

8. This report may contain comparative cost estimates for the purpose of evaluating alternative foundation schemes. These estimates may also involve approximate quantity evaluations. It should be noted that quantity estimates may not be accurate enough for construction bids. Since DTE has no control over labor and materials cost and design, the estimates of construction costs have been made on the basis of experience. DTE does not guarantee the accuracy of cost estimates as compared to contractor's bids for construction costs.

0644089821

NOTICE OF INTENT

New York State Department of Environmental Conservation
Division of Water
625 Broadway, 4th Floor
Albany, New York 12233-3505

NYR [ ][ ][ ][ ][ ][ ]
(for DEC use only)

Stormwater Discharges Associated with Construction Activity Under State Pollutant Discharge Elimination System (SPDES) General Permit # GP-0-20-001

All sections must be completed unless otherwise noted. Failure to complete all items may result in this form being returned to you, thereby delaying your coverage under this General Permit. Applicants must read and understand the conditions of the permit and prepare a Stormwater Pollution Prevention Plan prior to submitting this NOI. Applicants are responsible for identifying and obtaining other DEC permits that may be required.

-IMPORTANT-

RETURN THIS FORM TO THE ADDRESS ABOVE

OWNER/OPERATOR MUST SIGN FORM

Owner/Operator Information

Owner/Operator (Company Name/Private Owner Name/Municipality Name)
Frank and Juli Vitiello

Owner/Operator Contact Person Last Name (NOT CONSULTANT)
Vitiello

Owner/Operator Contact Person First Name
Frank

Owner/Operator Mailing Address
475 Pudding Street

City
Carmel

State
NY

Zip
10512-

Phone (Owner/Operator)
845-225-0395

Fax (Owner/Operator)
- -

Email (Owner/Operator)
vitiellos@gmail.com

FED TAX ID
- (not required for individuals)

Project Site Information

Project/Site Name
Vitiello Residence

Street Address (NOT P.O. BOX)
475 Pudding Street

Side of Street
○ North ○ South ● East ○ West

City/Town/Village (THAT ISSUES BUILDING PERMIT)
Town of Kent

State: NY
Zip: 10512-
County: Putnam
DEC Region: 3

Name of Nearest Cross Street
Gordon Road

Distance to Nearest Cross Street (Feet)
170

Project In Relation to Cross Street
● North ○ South ○ East ○ West

Tax Map Numbers
Section-Block-Parcel
31-1-32

Tax Map Numbers

1. Provide the Geographic Coordinates for the project site. To do this, go to the NYSDEC Stormwater Interactive Map on the DEC website at:

https://gisservices.dec.ny.gov/gis/stormwater/

Zoom into your Project Location such that you can accurately click on the centroid of your site. Once you have located the centroid of your project site, go to the bottom right hand corner of the map for the X, Y coordinates. Enter the coordinates into the boxes below. For problems with the interactive map use the help function.

X Coordinates (Easting)
-73.767
Ex. -73.749

Y Coordinates (Northing)
41.451
Ex. 42.652

2. What is the nature of this construction project?

○ New Construction

● Redevelopment with increase in impervious area

○ Redevelopment with no increase in impervious area

3. Select the predominant land use for both pre and post development conditions.
SELECT ONLY ONE CHOICE FOR EACH

Pre-Development Existing Land Use	Post-Development Future Land Use
○ FOREST	● SINGLE FAMILY HOME
○ PASTURE/OPEN LAND	○ SINGLE FAMILY SUBDIVISION
○ CULTIVATED LAND	○ TOWN HOME RESIDENTIAL
● SINGLE FAMILY HOME	○ MULTIFAMILY RESIDENTIAL
○ SINGLE FAMILY SUBDIVISION	○ INSTITUTIONAL/SCHOOL
○ TOWN HOME RESIDENTIAL	○ INDUSTRIAL
○ MULTIFAMILY RESIDENTIAL	○ COMMERCIAL
○ INSTITUTIONAL/SCHOOL	○ MUNICIPAL
○ INDUSTRIAL	○ ROAD/HIGHWAY
○ COMMERCIAL	○ RECREATIONAL/SPORTS FIELD
○ ROAD/HIGHWAY	○ BIKE PATH/TRAIL
○ RECREATIONAL/SPORTS FIELD	○ LINEAR UTILITY (water, sewer, gas, etc.)
○ BIKE PATH/TRAIL	○ PARKING LOT
○ LINEAR UTILITY	○ CLEARING/GRADING ONLY
○ PARKING LOT	○ DEMOLITION, NO REDEVELOPMENT
○ OTHER	○ WELL DRILLING ACTIVITY *(Oil, Gas, etc.)
	○ OTHER

Number of Lots

***Note:** for gas well drilling, non-high volume hydraulic fractured wells only

4. In accordance with the larger common plan of development or sale, enter the total project site area; the total area to be disturbed; existing impervious area to be disturbed (for redevelopment activities); and the future impervious area constructed within the disturbed area. (Round to the nearest tenth of an acre.)

Total Site Area	Total Area To Be Disturbed	Existing Impervious Area To Be Disturbed	Future Impervious Area Within Disturbed Area
32.5	0.9	0.1	0.2

5. Do you plan to disturb more than 5 acres of soil at any one time? ○ Yes ● No

6. Indicate the percentage of each Hydrologic Soil Group(HSG) at the site.

A	B	C	D
0 %	95 %	0 %	5 %

7. Is this a phased project? ○ Yes ● No

8. Enter the planned start and end dates of the disturbance activities.

Start Date 09/01/2020 – End Date 08/31/2021

9. Identify the nearest surface waterbody(ies) to which construction site runoff will discharge.

Name

NYSDEC Wetland OL-51

9a. Type of waterbody identified in Question 9?

- ○ Wetland / State Jurisdiction On Site (Answer 9b)
- ⊘ Wetland / State Jurisdiction Off Site
- ○ Wetland / Federal Jurisdiction On Site (Answer 9b)
- ○ Wetland / Federal Jurisdiction Off Site
- ○ Stream / Creek On Site
- ○ Stream / Creek Off Site
- ○ River On Site
- ○ River Off Site
- ○ Lake On Site
- ○ Lake Off Site
- ○ Other Type On Site
- ○ Other Type Off Site

9b. How was the wetland identified?

- ○ Regulatory Map
- ○ Delineated by Consultant
- ○ Delineated by Army Corps of Engineers
- ○ Other (identify)

10. Has the surface waterbody(ies) in question 9 been identified as a 303(d) segment in Appendix E of GP-0-20-001? ○ Yes ● No

11. Is this project located in one of the Watersheds identified in Appendix C of GP-0-20-001? ● Yes ○ No

12. Is the project located in one of the watershed areas associated with AA and AA-S classified waters? ● Yes ○ No
If no, skip question 13.

13. Does this construction activity disturb land with no existing impervious cover and where the Soil Slope Phase is identified as an E or F on the USDA Soil Survey? ○ Yes ● No
If Yes, what is the acreage to be disturbed?

14. Will the project disturb soils within a State regulated wetland or the protected 100 foot adjacent area? ○ Yes ● No

15. Does the site runoff enter a separate storm sewer system (including roadside drains, swales, ditches, culverts, etc)? ○ Yes ○ No ● Unknown

16. What is the name of the municipality/entity that owns the separate storm sewer system?

Town of Kent

17. Does any runoff from the site enter a sewer classified as a Combined Sewer? ○ Yes ● No ○ Unknown

18. Will future use of this site be an agricultural property as defined by the NYS Agriculture and Markets Law? ○ Yes ● No

19. Is this property owned by a state authority, state agency, federal government or local government? ○ Yes ● No

20. Is this a remediation project being done under a Department approved work plan? (i.e. CERCLA, RCRA, Voluntary Cleanup Agreement, etc.) ○ Yes ● No

21. Has the required Erosion and Sediment Control component of the SWPPP been developed in conformance with the current NYS Standards and Specifications for Erosion and Sediment Control (aka Blue Book)? ● Yes ○ No

22. Does this construction activity require the development of a SWPPP that includes the post-construction stormwater management practice component (i.e. Runoff Reduction, Water Quality and Quantity Control practices/techniques)? ○ Yes ● No
If No, skip questions 23 and 27-39.

23. Has the post-construction stormwater management practice component of the SWPPP been developed in conformance with the current NYS Stormwater Management Design Manual? ○ Yes ○ No

24. The Stormwater Pollution Prevention Plan (SWPPP) was prepared by:

- ☑ **Professional Engineer (P.E.)**
- ○ **Soil and Water Conservation District (SWCD)**
- ○ **Registered Landscape Architect (R.L.A)**
- ○ **Certified Professional in Erosion and Sediment Control (CPESC)**
- ○ **Owner/Operator**
- ○ **Other**

SWPPP Preparer
Insite Engineering

Contact Name (Last, Space, First)
Watson, John M., P.E.

Mailing Address
3 Garrett Place

City
Carmel

State NY Zip 10512 -

Phone 845 - 225 - 9690

Fax 845 - 225 - 9717

Email
jwatson@insite-eng.com

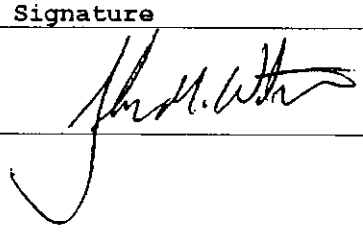
SWPPP Preparer Certification

I hereby certify that the Stormwater Pollution Prevention Plan (SWPPP) for this project has been prepared in accordance with the terms and conditions of the GP-0-20-001. Furthermore, I understand that certifying false, incorrect or inaccurate information is a violation of this permit and the laws of the State of New York and could subject me to criminal, civil and/or administrative proceedings.

First Name John **MI** M

Last Name Watson, P.E.

Signature



Date 09 / 04 / 2020

25. Has a construction sequence schedule for the planned management practices been prepared? ● Yes ○ No

26. Select all of the erosion and sediment control practices that will be employed on the project site:

Temporary Structural

- ○ Check Dams
- ○ Construction Road Stabilization
- ◉ Dust Control
- ○ Earth Dike
- ○ Level Spreader
- ○ Perimeter Dike/Swale
- ○ Pipe Slope Drain
- ○ Portable Sediment Tank
- ○ Rock Dam
- ○ Sediment Basin
- ○ Sediment Traps
- ◉ Silt Fence
- ◉ Stabilized Construction Entrance
- ○ Storm Drain Inlet Protection
- ○ Straw/Hay Bale Dike
- ○ Temporary Access Waterway Crossing
- ○ Temporary Stormdrain Diversion
- ○ Temporary Swale
- ○ Turbidity Curtain
- ○ Water bars

Biotechnical

- ○ Brush Matting
- ○ Wattling

Vegetative Measures

- ○ Brush Matting
- ○ Dune Stabilization
- ○ Grassed Waterway
- ◉ Mulching
- ○ Protecting Vegetation
- ○ Recreation Area Improvement
- ◉ Seeding
- ○ Sodding
- ○ Straw/Hay Bale Dike
- ○ Streambank Protection
- ○ Temporary Swale
- ◉ Topsoiling
- ○ Vegetating Waterways

Permanent Structural

- ○ Debris Basin
- ○ Diversion
- ○ Grade Stabilization Structure
- ○ Land Grading
- ○ Lined Waterway (Rock)
- ○ Paved Channel (Concrete)
- ○ Paved Flume
- ◉ Retaining Wall
- ○ Riprap Slope Protection
- ○ Rock Outlet Protection
- ○ Streambank Protection

Other

Post-construction Stormwater Management Practice (SMP) Requirements

Important: Completion of Questions 27-39 is not required if response to Question 22 is No.

27. Identify all site planning practices that were used to prepare the final site plan/layout for the project.

- O **Preservation of Undisturbed Areas**
- O **Preservation of Buffers**
- O **Reduction of Clearing and Grading**
- O **Locating Development in Less Sensitive Areas**
- O **Roadway Reduction**
- O **Sidewalk Reduction**
- O **Driveway Reduction**
- O **Cul-de-sac Reduction**
- O **Building Footprint Reduction**
- O **Parking Reduction**

27a. Indicate which of the following soil restoration criteria was used to address the requirements in Section 5.1.6("Soil Restoration") of the Design Manual (2010 version).

- O All disturbed areas will be restored in accordance with the Soil Restoration requirements in Table 5.3 of the Design Manual (see page 5-22).
- O Compacted areas were considered as impervious cover when calculating the **WQv Required**, and the compacted areas were assigned a post-construction Hydrologic Soil Group (HSG) designation that is one level less permeable than existing conditions for the hydrology analysis.

28. Provide the total Water Quality Volume (WQv) required for this project (based on final site plan/layout).

Total WQv Required

[ ][ ][ ].[ ][ ][ ] **acre-feet**

29. Identify the RR techniques (Area Reduction), RR techniques(Volume Reduction) and Standard SMPs with RRv Capacity in Table 1 (See Page 9) that were used to reduce the Total WQv Required(#28).

Also, provide in Table 1 the total impervious area that contributes runoff to each technique/practice selected. For the Area Reduction Techniques, provide the total contributing area (includes pervious area) and, if applicable, the total impervious area that contributes runoff to the technique/practice.

Note: Redevelopment projects shall use Tables 1 and 2 to identify the SMPs used to treat and/or reduce the WQv required. If runoff reduction techniques will not be used to reduce the required WQv, skip to question 33a after identifying the SMPs.

Table 1 - Runoff Reduction (RR) Techniques and Standard Stormwater Management Practices (SMPs)

RR Techniques (Area Reduction)	Total Contributing Area (acres)		Total Contributing Impervious Area (acres)	
○ Conservation of Natural Areas (RR-1)	___ . ___	and/or	___ . ___	
○ Sheetflow to Riparian Buffers/Filters Strips (RR-2)	___ . ___	and/or	___ . ___	
○ Tree Planting/Tree Pit (RR-3)	___ . ___	and/or	___ . ___	
○ Disconnection of Rooftop Runoff (RR-4)	___ . ___	and/or	___ . ___	

RR Techniques (Volume Reduction)	
○ Vegetated Swale (RR-5)	___ . ___
○ Rain Garden (RR-6)	___ . ___
○ Stormwater Planter (RR-7)	___ . ___
○ Rain Barrel/Cistern (RR-8)	___ . ___
○ Porous Pavement (RR-9)	___ . ___
○ Green Roof (RR-10)	___ . ___

Standard SMPs with RRv Capacity	
○ Infiltration Trench (I-1)	___ . ___
○ Infiltration Basin (I-2)	___ . ___
○ Dry Well (I-3)	___ . ___
○ Underground Infiltration System (I-4)	___ . ___
○ Bioretention (F-5)	___ . ___
○ Dry Swale (O-1)	___ . ___

Standard SMPs	
○ Micropool Extended Detention (P-1)	___ . ___
○ Wet Pond (P-2)	___ . ___
○ Wet Extended Detention (P-3)	___ . ___
○ Multiple Pond System (P-4)	___ . ___
○ Pocket Pond (P-5)	___ . ___
○ Surface Sand Filter (F-1)	___ . ___
○ Underground Sand Filter (F-2)	___ . ___
○ Perimeter Sand Filter (F-3)	___ . ___
○ Organic Filter (F-4)	___ . ___
○ Shallow Wetland (W-1)	___ . ___
○ Extended Detention Wetland (W-2)	___ . ___
○ Pond/Wetland System (W-3)	___ . ___
○ Pocket Wetland (W-4)	___ . ___
○ Wet Swale (O-2)	___ . ___

Table 2 - Alternative SMPs
(DO NOT INCLUDE PRACTICES BEING USED FOR PRETREATMENT ONLY)

Alternative SMP	Total Contributing Impervious Area (acres)
○ Hydrodynamic	
○ Wet Vault	
○ Media Filter	
○ Other	

Provide the name and manufacturer of the Alternative SMPs (i.e. proprietary practice(s)) being used for WQv treatment.

Name

Manufacturer

Note: Redevelopment projects which do not use RR techniques, shall use questions 28, 29, 33 and 33a to provide SMPs used, total WQv required and total WQv provided for the project.

30. Indicate the Total RRv provided by the RR techniques (Area/Volume Reduction) and Standard SMPs with RRv capacity identified in question 29.

Total RRv provided

. acre-feet

31. Is the Total RRv provided (#30) greater than or equal to the total WQv required (#28).

○ Yes ○ No

If Yes, go to question 36.
If No, go to question 32.

32. Provide the Minimum RRv required based on HSG.
[Minimum RRv Required = (P)(0.95)(Ai)/12, Ai=(S)(Aic)]

Minimum RRv Required

. acre-feet

32a. Is the Total RRv provided (#30) greater than or equal to the Minimum RRv Required (#32)?

○ Yes ○ No

If Yes, go to question 33.
Note: Use the space provided in question #39 to summarize the specific site limitations and justification for not reducing 100% of WQv required (#28). A detailed evaluation of the specific site limitations and justification for not reducing 100% of the WQv required (#28) must also be included in the SWPPP.
If No, sizing criteria has not been met, so NOI can not be processed. SWPPP preparer must modify design to meet sizing criteria.

33. Identify the Standard SMPs in Table 1 and, if applicable, the Alternative SMPs in Table 2 that were used to treat the remaining total WQv(=Total WQv Required in 28 - Total RRv Provided in 30).

Also, provide in Table 1 and 2 the total impervious area that contributes runoff to each practice selected.

Note: Use Tables 1 and 2 to identify the SMPs used on Redevelopment projects.

33a. Indicate the Total WQv provided (i.e. WQv treated) by the SMPs identified in question #33 and Standard SMPs with RRv Capacity identified in question 29.

WQv Provided

[ ][ ][ ].[ ][ ][ ] **acre-feet**

Note: For the standard SMPs with RRv capacity, the WQv provided by each practice = the WQv calculated using the contributing drainage area to the practice - RRv provided by the practice. (See Table 3.5 in Design Manual)

34. Provide the sum of the Total RRv provided (#30) and the WQv provided (#33a). [ ][ ][ ].[ ][ ][ ]

35. Is the sum of the RRv provided (#30) and the WQv provided (#33a) greater than or equal to the total WQv required (#28)? ○ **Yes** ○ **No**

If Yes, go to question 36.
If No, sizing criteria has not been met, so NOI can not be processed. SWPPP preparer must modify design to meet sizing criteria.

36. Provide the total Channel Protection Storage Volume (CPv) required and provided or select waiver (36a), if applicable.

CPv Required

[ ][ ][ ].[ ][ ][ ] **acre-feet**

CPv Provided

[ ][ ][ ].[ ][ ][ ] **acre-feet**

36a. The need to provide channel protection has been waived because:

○ Site discharges directly to tidal waters or a fifth order or larger stream.

○ Reduction of the total CPv is achieved on site through runoff reduction techniques or infiltration systems.

37. Provide the Overbank Flood (Qp) and Extreme Flood (Qf) control criteria or select waiver (37a), if applicable.

Total Overbank Flood Control Criteria (Qp)

Pre-Development

[ ][ ][ ].[ ][ ][ ] CFS

Post-development

[ ][ ][ ].[ ][ ][ ] CFS

Total Extreme Flood Control Criteria (Qf)

Pre-Development

[ ][ ][ ].[ ][ ][ ] CFS

Post-development

[ ][ ][ ].[ ][ ][ ] CFS

37a. The need to meet the Qp and Qf criteria has been waived because:

- O Site discharges directly to tidal waters or a fifth order or larger stream.
- O Downstream analysis reveals that the Qp and Qf controls are not required

38. Has a long term Operation and Maintenance Plan for the post-construction stormwater management practice(s) been developed? O Yes O No

If Yes, Identify the entity responsible for the long term Operation and Maintenance

39. Use this space to summarize the specific site limitations and justification for not reducing 100% of WQv required(#28). (See question 32a) This space can also be used for other pertinent project information.

40. Identify other DEC permits, existing and new, that are required for this project/facility.

- ○ Air Pollution Control
- ○ Coastal Erosion
- ○ Hazardous Waste
- ○ Long Island Wells
- ○ Mined Land Reclamation
- ○ Solid Waste
- ○ Navigable Waters Protection / Article 15
- ○ Water Quality Certificate
- ○ Dam Safety
- ○ Water Supply
- ○ Freshwater Wetlands/Article 24
- ○ Tidal Wetlands
- ○ Wild, Scenic and Recreational Rivers
- ○ Stream Bed or Bank Protection / Article 15
- ○ Endangered or Threatened Species(Incidental Take Permit)
- ○ Individual SPDES
- ○ SPDES Multi-Sector GP [N][Y][R][ ][ ][ ][ ][ ][ ]
- ○ Other [ ]
- ◉ None

41. Does this project require a US Army Corps of Engineers Wetland Permit? ○ Yes ● No
If Yes, Indicate Size of Impact. [ ][ ][ ][ ][ ].[ ]

42. Is this project subject to the requirements of a regulated, traditional land use control MS4? ● Yes ○ No
(If No, skip question 43)

43. Has the "MS4 SWPPP Acceptance" form been signed by the principal executive officer or ranking elected official and submitted along with this NOI? ● Yes ○ No

44. If this NOI is being submitted for the purpose of continuing or transferring coverage under a general permit for stormwater runoff from construction activities, please indicate the former SPDES number assigned. [N][Y][R][ ][ ][ ][ ][ ][ ]

Owner/Operator Certification

I have read or been advised of the permit conditions and believe that I understand them. I also understand that, under the terms of the permit, there may be reporting requirements. I hereby certify that this document and the corresponding documents were prepared under my direction or supervision. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations. I further understand that coverage under the general permit will be identified in the acknowledgment that I will receive as a result of submitting this NOI and can be as long as sixty (60) business days as provided for in the general permit. I also understand that, by submitting this NOI, I am acknowledging that the SWPPP has been developed and will be implemented as the first element of construction, and agreeing to comply with all the terms and conditions of the general permit for which this NOI is being submitted.

Print First Name

Frank

MI

Print Last Name

Vitiello

Owner/Operator Signature

Date

09/06/2020

Reply all | Delete | Junk | Block | ...

Re: GADF

AM

alessandro mazzotta <amazz2@yahoo.com>

Fri 9/11/2020 1:48 PM

To: Planning Kent

TOWN OF KENT NOTICE

THIS EMAIL IS FROM AN EXTERNAL SENDER!
DO NOT click links, DO NOT open attachments, DO NOT forward if you were not expecting this email or if it seems suspicious in any way! REMEMBER: NEVER provide your user ID or password to anyone for any reason!

Hi Vera

Please refund the balance of our escrow funds related to the GADF site plan review.

Thank you.

Alex

Sent from Yahoo Mail for iPhone

On Friday, September 11, 2020, 11:16 AM, Planning Kent <planningkent@townofkentny.gov> wrote:

> Attached is a copy of an invoice submitted in August 2020 for your project which I may have sent already.
>
> Please send another note requesting that your escrow be refunded.
>
> Thanks very much.
>
> Vera Patterson
> Town of Kent
> Planning Board Secretary
> 25 Sybil's Crossing
> Kent, NY 10512
> planningkent@townofkentny.gov
> 845-225-7892 (T)

Town of Kent Planning Board
25 Sybil's Crossing
Tel: 845-225-7802

email: planningkent@townofkentny.gov
Kent, NY 10512
Fax: 845-306-5283

Memorandum

DATE: **June 18, 2020**

TO: **Finance Department**

CC:

FROM: **Vera Patterson**

Re: **GADF LLC - TM: 12.17-1-9**

Please find attached copies of supporting documents for an Amended Site Plan/Change of Use application. However, this should be treated as a new project. Also attached are the following checks:

M & T Bank Check 1011 , dated 6/16/20 in the amount of $ 150.00 – Amended Site Plan
M & T Bank Check 1012, dated 6/16/20 in the amount of $1,000.00 – Review/Escrow Fees

Thanks very much.

RECEIPT

TOWN OF KENT-PLANNING BOARD
25 SYBILS CROSSING
KENT LAKES, NY 10512
(845) 225-7802

3326

DATE June 16, 202

RECEIVED FROM D. Mazzotta $ 150.00 ck

One Hundred fifty and 00/100 DOLLARS

FOR Amended Site Plan TM: 12.17-1-9 Change of Plan

AMOUNT OF ACCOUNT		
THIS PAYMENT		
BALANCE DUE		

☐ CASH ☑ CHECK 1611 ☐ M.O. BY V. Patterson

THANK YOU

RECEIPT

TOWN OF KENT-PLANNING BOARD
25 SYBILS CROSSING
KENT LAKES, NY 10512
(845) 225-7802

3327

DATE June 16, 202

RECEIPT FROM D. Mazzotta $ 1,000.00 ck

One Thousand and 00/100 DOLLARS

FOR Escrow Rev. Fee - TM: 12.17-1-9

AMOUNT OF ACCOUNT		
THIS PAYMENT		
BALANCE DUE		

☐ CASH ☑ CHECK 1612 ☐ M.O. BY V. Patterson

THANK YOU

GADF, LLC
14 RESERVOIR RD
BREWSTER, NY 10509

1011

DATE 6/16/20 10-4/220

PAY TO THE ORDER OF Town of Kent $ 150.00

one hundred fifty 00/100 DOLLARS

M&T Bank

FOR Application Fee Site Plan TM 12.17-1-9

⑈001011⑈ ⑆022000046⑆ 9876817355⑈

GADF, LLC
14 RESERVOIR RD
BREWSTER, NY 10509

1012

DATE 6/16/20 10-4/220

PAY TO THE ORDER OF Town of Kent $ 1000.00

one Thousand 00/100 DOLLARS

M&T Bank

FOR Escrow - TM: 12.17-1-9

Date		Deposits	Fees	Balance
6/23/2020	Check #1012	1,000.00		1,000.00
08/26/20	Rohde-29657-2466		(140.00)	860.00
				860.00

GADF LLC
1088 ROUTE 52
Carmel, New York 10512
(845) 797-6882
May 19, 2020

Town of Kent Planning Board,

GADF LLC requests that we be placed on the next Planning Board Agenda. GADF owns 1088 Route 52. This property is adjacent to the Kent Center. Prior to the COVID crisis, a certificate of occupancy was granted to Coldwell Banker to occupy 1600 square feet of the building space to operate a real estate brokerage office (front of the building that faces Route 52). In the rear of the building is an 800 square foot, 2- bedroom apartment. This apartment is unoccupied. We would like the Planning Board to consider and approve the 2- bedroom apartment as a permitted accessory use of the building per Town Code Section 77-21(B)(5).

We believe that the commercial rental market will continue to be suppressed for some time while the COVID crisis persists. However, the residential market may be able to recover a little quicker.

We look forward to discussing this matter with you at the upcoming meeting.

Thank you.

Alex Mazzotta

Very truly yours,

GADF LLC
Alex Mazzotta

TOWN OF KENT PLANNING BOARD
SITE PLAN CHECKLIST

JUN 17 2020

Planning Department
Town of Kent

APPLICANT NAME: GADF, LLC

ADDRESS: 114 Reservoir Road, Brewster, NY 10509

CONTACT TELEPHONE NUMBER: 914-582-8183

TM: 12.17-1-9

The following preliminary information must be included on the site plan. Please either check box as completed or indicate N/A (not applicable).

1. X The dimensions of all property lines
2. X Identify scale used
3. X Name of all adjacent roads and driveway location
4. N/A Sight distances if new curb cut is requested
5. N/A Easements for utilities including overhead
6. X All existing structures (including pools) shown and labeled as to their use and the distance from proposed structure and property lines
7. N/A Distance from the proposed structure to ALL property lines
8. X Completed bulk zoning table
9. N/A Location of any wetland, stream, lake or body of water within 100 feet of the property line.
10. X Location of septic system (including 100% expansion area)
11. X Location of well head
12. N/A Pre and post-construction topography (grading plan)
13. N/A Total limit of disturbance line
14. N/A Area(s) of disturbance where slopes are greater than 15%
15. N/A Total area of disturbance calculation (in square feet)
16. N/A Erosion and sediment control plan (if area of disturbance is greater than 5,000 square feet)
17. N/A Cost estimate (breakdown) to implement erosion and sediment control plan
18. X KNOX box system (if commercial property)

Check list completed by:

Jeffrey J. Contelmo, P.E.
(Print or type name here)
Insite Engineering, Surveying & Landscape Architecture, P.C.

(Signature)

Senior Principal Engineer
(Print or type Title here)

6/18/20
(Date)

DO NOT WRITE BELOW THIS LINE (OFFICIAL USE)

Plans Date Stamped: _____ Reviewer: _____ Date: _____

Notes: _____

TOWN OF KENT, NEW YORK

mo/date/year

JUN 17 2020

Planning Department
Town of Kent

GADF LLC
1088 ROUTE 52
Carmel, New York 10512
(845) 797-6882
June 18, 2020

Town of Kent Planning Board,

Per our telephone conference call on May 26, 2020 I present the following for the Planning Board's consideration for its July 2020 meeting.

1. A site plan of 1088 Route 52 as prepared by Insite Engineering.
2. A floor plan detailing the proposed uses of the subject building.
3. Details of a proposed new freestanding sign. We are confident the sign is in conformance with Section 77-37(A)(2) as it is approximately 16.5 square feet which falls below the majority size of 20 square feet. The sign will meet the maximum height requirement of 8 feet.
4. We are also requesting approval of an accessory usage that is permitted under Section 77-21(B)(5); a residential unit located above a nonresidential use. This is denoted as Apartment on the Building Floor Plan.

We look forward to discussing this matter with you at the upcoming meeting.

Thank you.

Very truly yours,

Alex Mazzotta

GADF LLC
Alex Mazzotta